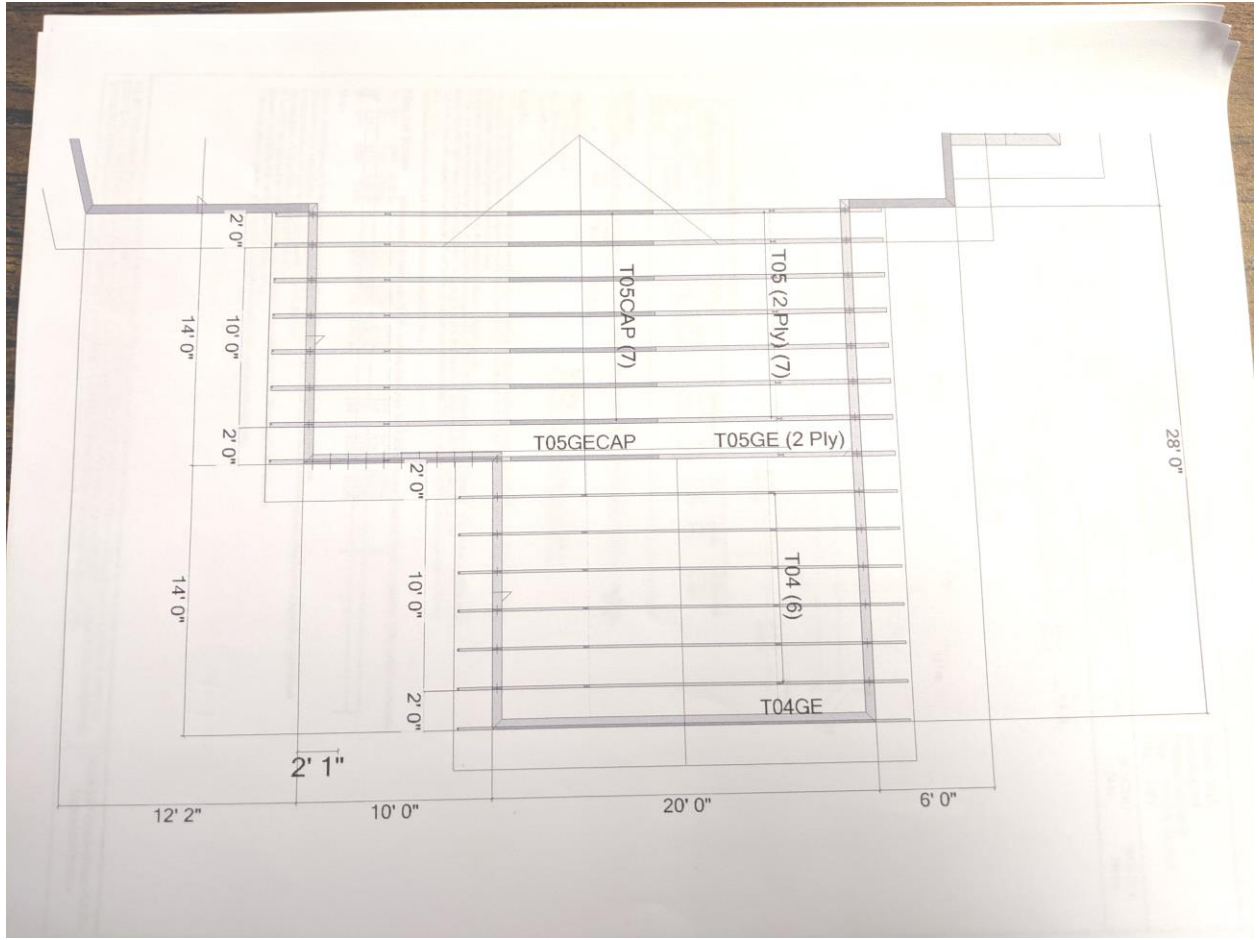
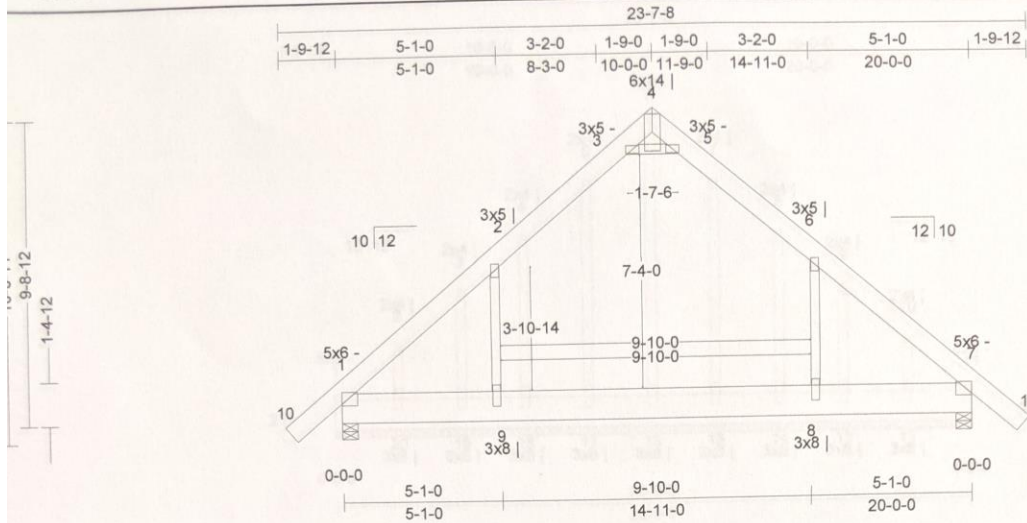


**Paranzino Brothers Auctions**  
**August 20<sup>th</sup>, 2019**  
**North Lima, OH**  
**Truss kit**  
**Available at public auction**



SPAN 20-0-0	PITCH 10/12	QTY 6	OHL 1-9-12	OHR 1-9-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/P 190 lb
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IRC 2012/	TC : 0.38 (3-4)	Vert TL: 0.3 in	L / 753	(8-9)	L / 240
Snow(Ps/Pg) 18/25	TPR 1-2007	BC : 0.57 (8-9)	Vert LL: 0.13 in	L / 999	(8-9)	L / 360
TCDL : 10 (make)	Rep Mfr: Yes	Web : 0.23 (3-5)	Horz TL: 0.01 in		7	
BCLL : 0	Lumber D.O.L. :115 %					
BCDL : 10						

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	6 in	1.86 in	1,573 lbs					67 lbs
7	1	6 in	1.86 in	1,573 lbs					

**Material**

TC: SP 2400/2.0 2 x 8  
 BC: SYP #1 2 x 12  
 Web: SYP #2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (17.9 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSI, Terrain B, Exposure (Ce=1.0), Risk Category II (I=1.00), Thermal (Ct=1.10), DOL = 1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2012 assuming slope = 10/12 and area supported = 47.25 ft<sup>2</sup>, DOL = 115 %.
- Minimum storage attic loading has been applied in accordance with IRC 301.5

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force) Only forces greater than 300lbs are shown in this table.

TC	1-2	0.296	-1,629 lbs	3-4	0.377	654 lbs	5-6	0.294	-1,090 lbs
	2-3	0.294	-1,090 lbs	4-5	0.377	654 lbs	6-7	0.296	-1,629 lbs
BC	7-8	0.178	1,002 lbs	8-9	0.569	1,002 lbs	9-1	0.178	1,002 lbs
Web	2-9	0.224	912 lbs	3-5	0.234	-1,763 lbs	6-8	0.224	912 lbs

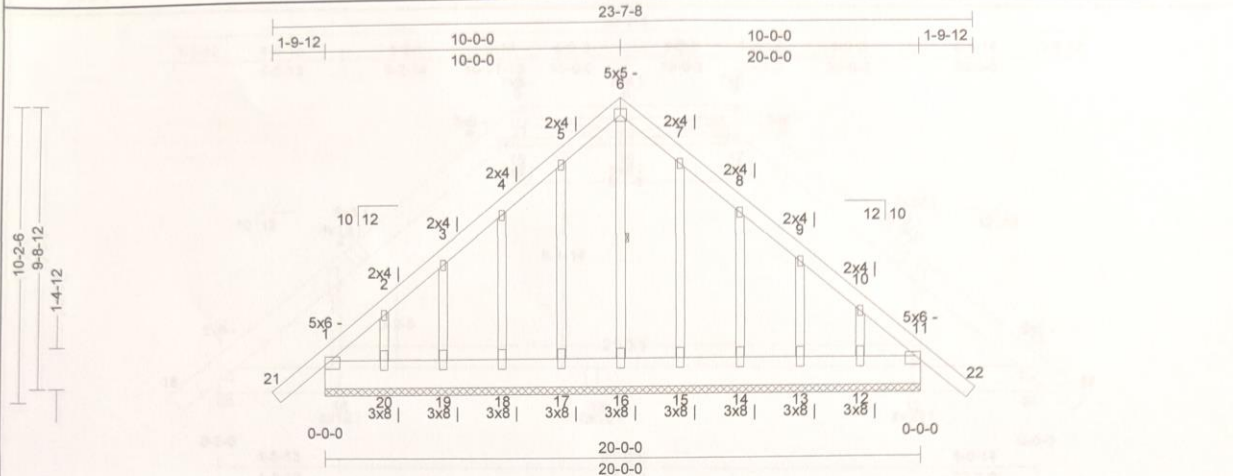
**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Attic floor area has been designed as a living area with 40 psf flo or live load and a 5 psf floor dead load, and the interior vertical wds and ceiling has been designed for a 5 psf dead load.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary
- A steep factor of 1.50 has been applied for this truss analysis.
- The 'SYP' label shown in the 'Material Summary' above indicates the new SP1B design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.
- Bottom chord in the open area of this truss meets L/360 for live load and L/240 for total load deflection criteria.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS' DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v  
 Eagle Metal Products

SPAN 20-0-0	PITCH 10/12	QTY 1	OHL 1-9-12	OHR 1-9-12	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 206 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General		CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IRC 2012/	TC : 0.08 (21-1)	Vert TL: 0 in	L / 999	12	L / 240
Snow (Ps/Pg) : 18/25	TPI 1-2007	BC : 0.00 (20-1)	Vert LL: 0 in	L / 999	12	L / 360
TCDL : 10 (rake)	Rep Mir: No	Web : 0.18 (5-17)	Horz TL: 0 in			
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grv Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		339 lbs	117 plf	-10 lbs	-26 lbs	-45 lbs	-45 lbs	-305 lbs

**Material**

TC: SP 2400/2.0 2 x 6  
 BC: SYP #1 2 x 12  
 Web: SYP #2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.  
 Web: One Midpoint Row: 6-16

**Loads**

- 1) This truss has been designed for the effects of balanced (17.9 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSI, Terrain B, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2012 assuming slope = 10/12 and area supported = 47.25 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force) Only forces greater than 300lbs are shown in this table.

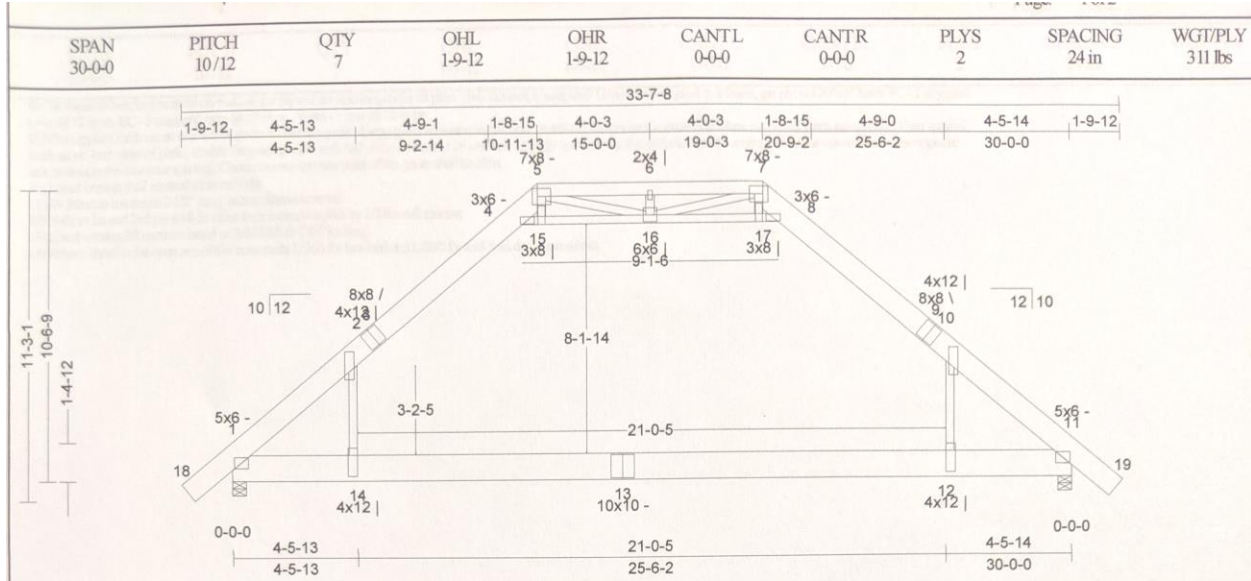
Member	Max CSI	Max Axial Force
TC		
BC		
Web		

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 3x8 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) Gable must be sheathed on one side or lateral bracing applied appropriately.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SYP design values effective June 1, 2013 were used.
- 9)  Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 12, 20 may need to be considered.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.6.32  
 Eagle Metal Products



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IRC 2012/	TC: 0.34 (1-2)	Vert TL: 0.84 in	L / 413	(12-13)	L / 240
Snow (PsPg) 20/25	TP1 1-2007	BC: 0.77 (12-14)	Vert LL: 0.43 in	L / 811	(12-13)	L / 360
TCDL: 10 (rake)	Rep Mir: Yes	Web: 0.35 (2-14)	Horz TL: 0.02 in		11	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	6 in	1.50 in	2,537 lbs					-73 lbs
11	1	6 in	1.50 in	2,537 lbs					

**Material**

TC: SP 2400/2.0 2 x 10 except  
 SP 2400/2.0 2 x 6: 5-7  
 BC: SYP#1 2 x 12  
 Web: SPF#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSI, Terrain B, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Minimum storage attic loading has been applied in accordance with IRC 301.5

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force) Only forces greater than 300lbs are shown in this table.

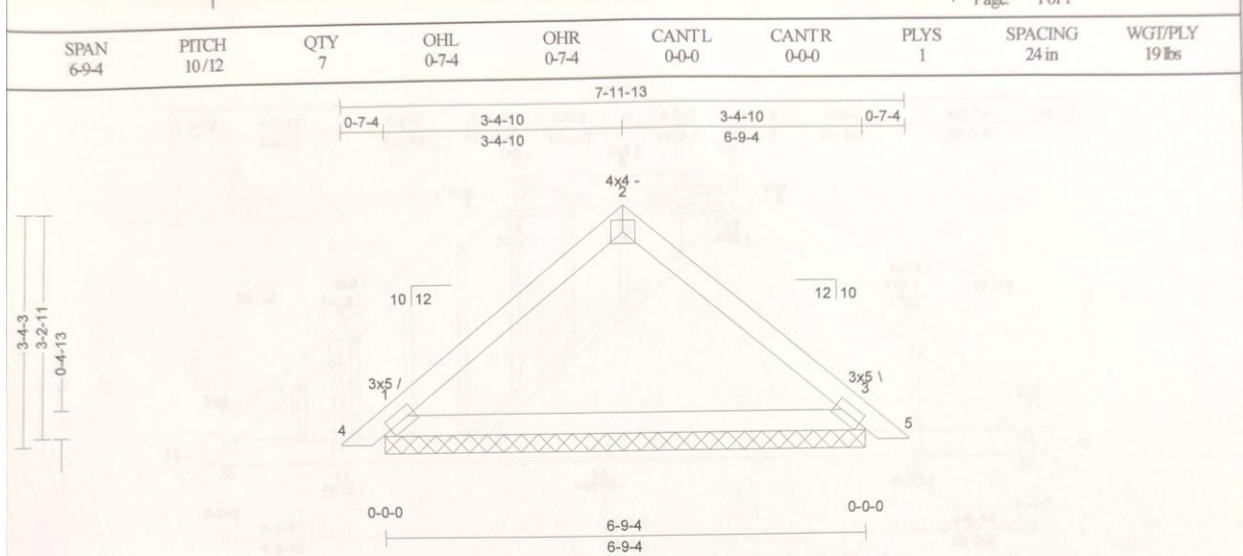
TC	1-2	0.343	-1,788 lbs	5-6	0.196	974 lbs	(67 lbs)	8-10	0.342	-1,022 lbs
	2-4	0.342	-1,022 lbs	6-7	0.196	974 lbs	(67 lbs)	10-11	0.343	-1,788 lbs
	4-5	0.262	-368 lbs	7-8	0.262	-368 lbs				
BC	11-12	0.589	988 lbs	12-14	0.769	988 lbs		14-1	0.589	988 lbs
Web	2-14	0.348	1,418 lbs	15-16	0.252	-1,705 lbs		8-17	0.217	-1,723 lbs
	4-15	0.217	-1,723 lbs	16-17	0.252	-1,705 lbs		10-12	0.348	1,418 lbs
	5-15	0.167	680 lbs	7-17	0.167	680 lbs				

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Attic floor area has been designed as a living area with 40 psf flo or live load and a 5 psf floor dead load, and the interior vertical webs and ceiling has been designed for a 5 psf dead load.
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral bracing shall be installed within 6" of each web panel point.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.6.  
 Eagle Metal Products



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IRC 2012/	TC : 0.21 (1-2)	Vet TL : 0.01 in	L / 999	(3-1)	L / 240
Snow (Ps/Pg) : 18/25	TPH 1-2007	BC : 0.04 (3-1)	Vet LL : 0 in	L / 999	3	L / 360
TCDL : 10 (rike)	Rep Mbr : Yes	Web : 0.00 (1)	Horz TL : 0 in			
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 2						

**Reaction**

Big Combo	Big Width	Max React	Ave React	Max Grv Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		566 lbs	189 plf	-154 lbs	-36 lbs	-102 lbs	-154 lbs	336 lbs

**Material**

TC: SPF#2 2 x 4  
 BC: SPF#2 2 x 4  
 Web:

**Bracing**

TC: Sheathed or Putins at 6-3-0, Putin design by Others.  
 BC: Sheathed or Putins at 10-0-0, Putin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (17.9 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce=1.0), Risk Category II (I=1.00), Thermal (Ct=1.10), DOL=1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2012 assuming slope = 10/12 and area supported = 15.97 ft<sup>2</sup>, DOL=115%.

**Member Forces**

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

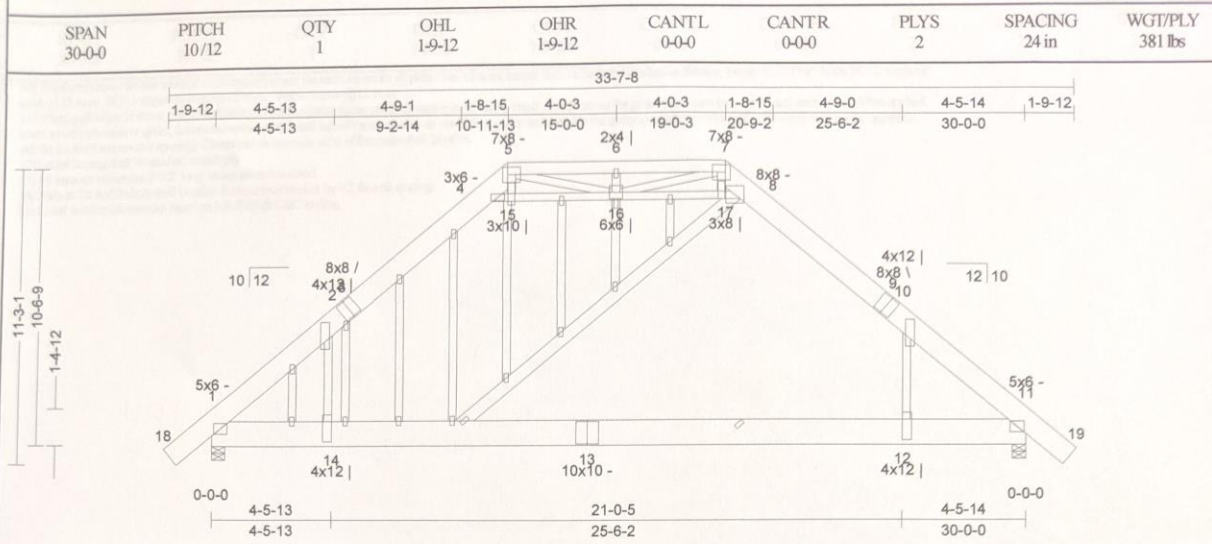
TC	1-2	0.208	-301 lbs	2-3	0.208	-301 lbs
BC						
Web						

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 48" OC, U.N.O.
- Attach gable webs with 3x5 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to ECSI-E3 published by the SBCEA.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 2 psf.
- A creep factor of 1.50 has been applied for this truss analysis.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.6.326  
 Eagle Metal Products



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IRC 2012/	TC : 0.39 (1-2)	Vert TL : 0.84 in	L/413	(12-13)	L/240
Snow (Ps/Pg) 20/25	TH 1-2007	BC : 0.88 (12-14)	Vert LL : 0.43 in	L/811	(12-13)	L/360
TCDL : 10 (take)	Rep Mfr : No	Web : 0.35 (2-14)	Horz TL : 0.02 in		11	
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

JT	Brg Combo	Brg Width	Rqd Brg Width	Max Rest	Max Grv Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	6 in	1.50 in	2,537 lbs					
11	1	6 in	1.50 in	2,537 lbs					-73 lbs

**Material**  
 TC: SP 2400/2.0 2x10 except  
 SP 2400/2.0 2x6 5-7  
 BC: SYP#1 2x12  
 Web: SPF #2 2x4

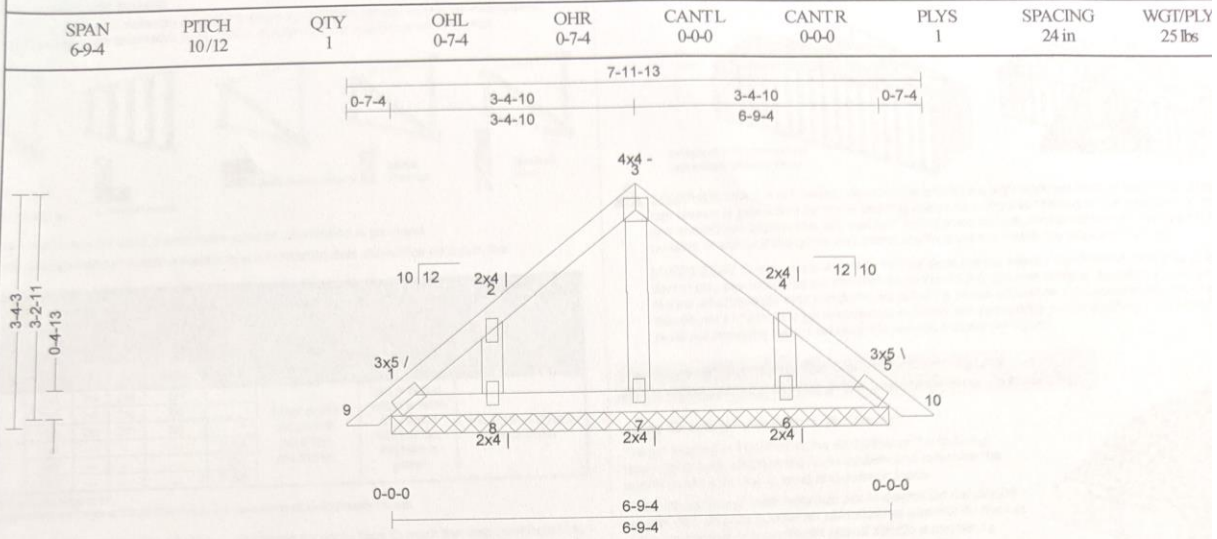
**Bracing**  
 TC: Sheathed or Putins at 6-3-0, Putin design by Others.  
 BC: Sheathed or Putins at 10-0-0, Putin design by Others.

**Loads**  
 1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.  
 2) This truss has been designed to account for the effects of ice dams forming at the eaves.  
 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60  
 4) Minimum storage attic loading has been applied in accordance with IRC 301.5

**Member Forces** Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force) Only forces greater than 300lbs are shown in this table.

TC	1-2	0.394	-1,788 lbs	5-6	0.223	974 lbs	(-67 lbs)	8-10	0.393	-1,022 lbs
	2-4	0.393	-1,022 lbs	6-7	0.223	974 lbs	(-67 lbs)	10-11	0.394	-1,788 lbs
	4-5	0.301	-368 lbs	7-8	0.301	-368 lbs				
BC	11-12	0.674	988 lbs	12-14	0.881	988 lbs		14-1	0.674	988 lbs
Web	2-14	0.348	1,418 lbs	15-16	0.252	-1,705 lbs		8-17	0.217	-1,723 lbs
	4-15	0.217	-1,723 lbs	16-17	0.252	-1,705 lbs		10-12	0.348	1,418 lbs
	5-15	0.167	680 lbs	7-17	0.167	680 lbs				

- Notes**
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
  - 2) Gable webs placed at 24" OC, U.N.O.
  - 3) Attach structural gable blocks with 2x4 2ga plates, U.N.O.
  - 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSHB3 published by the SBCA.
  - 5) Attic floor area has been designed as a living area with 40 psf flo or live load and a 5 psf floor dead load, and the interior vertical webs and ceiling has been designed for a 5 psf dead load.
  - 6) Provide adequate drainage to prevent ponding.
  - 7) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
  - 8) A comp factor of 1.50 has been applied for this truss analysis.
  - 9) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IRC 2012/	TC : 0.06 (2-3)	Vert TL : 0 in	L/999	5	L/240
Snow(Ps/Pg) : 18/25	TP1-2007	BC : 0.02 (7-8)	Vert LL : 0 in	L/999	5	L/360
TCDL : 10 (rake)	Rep Mbr : No	Web : 0.02 (2-8)	Horz TL : 0 in			
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

Reaction	Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
	1		224 lbs	142 plf	-25 lbs	-1 lbs	-38 lbs	-38 lbs	-67 lbs

**Material**  
 TC: SPF #2 2 x 4  
 BC: SPF #2 2 x 4  
 Web: SPF #2 2 x 4

**Bracing**  
 TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

- Loads**
- 1) This truss has been designed for the effects of balanced (17.9 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
  - 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
  - 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Partial, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
  - 4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2012 assuming slope = 10/12 and area supported = 15.97 ft<sup>2</sup>, DOL = 115 %.

**Member Forces** Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force) Only forces greater than 300lbs are shown in this table.

TC	
BC	
Web	

- Notes**
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
  - 2) Gable requires continuous bottom chord bearing.
  - 3) Gable webs placed at 24" OC, U.N.O.
  - 4) Attach gable webs w/ 2x4 20ga plates, U.N.O.
  - 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BC5H3 published by the SBCA.
  - 6) A creep factor of 1.50 has been applied for this truss analysis.
  - 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 5 may need to be considered.
  - 8) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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 Eagle Metal P